## EQUADIFF 3

## Jean Descloux <br> Finite elements and numerical stability

In: Miloš Ráb and Jaromír Vosmanský (eds.): Proceedings of Equadiff III, Ord Czechoslovak Conference on Differential Equations and Their Applications. Brno, Czechoslovakia, August 28 September 1, 1972. Univ. J. E. Purkyně - Přírodovědecká fakulta, Brno, 1973. Folia Facultatis Scientiarum Naturalium Universitatis Purkynianae Brunensis. Seria Monographic, Tomus I. pp. 21--29.

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## FINITE ELEMENTS AND NUMERICAL STABILITY

by JEAN DESCLOUX

## 1. INTRODUCTION

We use the following notations. When applied to an element $x$ of $\boldsymbol{R}^{N},\|$.$\| is$ a vector norm; when applied to a real square matrix $A$ of order $N,\|$.$\| is the matrix$ norm subordinate to the vector norm $\|$. $\|$, i.e. $\|A\|=\sup \|A x\| /\|x\|$. Here $\|$. $\|$ will be either the euclidean norm $\|\cdot\|_{2}$ or the uniform norm $\|\cdot\|_{\infty}$ defined by $\|x\|_{2}=$ $=\left(\sum_{i=1}^{N} x_{i}{ }^{2}\right)^{\frac{1}{2}}$ and $\|x\|_{\infty}=\max _{i=1, \ldots, N}\left|x_{i}\right|$.

Consider the regular linear system $A x=b$ and the perturbed system $(A+\delta A) x=$ $=b+\delta b$ with solutions $x_{0}$ and $x_{0}+\delta x$. Supposing $\delta A$ and $\delta b$ "small" and neglecting terms of "higher order" one gets the approximate relation (see [1]):

$$
\begin{equation*}
\frac{\|\delta x\|}{\left\|x_{0}\right\|} \leqq C(A)\left\{\frac{\|\delta b\|}{\|b\|}+\frac{\|\delta A\|}{\|A\|}\right\} \tag{1}
\end{equation*}
$$

where $C(A)=\|A\|\left\|A^{-1}\right\|$ is the condition number of $A$ with respect to $\|$.$\| .$ Suppose we use a computer to solve numerically the system $A x=b$, for example, by Gauss elimination. Because of round-off errors the solution $x_{1}$ produced by the computer will differ from the exact solution $x_{0}$; using the inverse round-off a nalysis (see [1]), one can show the existence of a "small" matrix $\delta A$ such that $x_{1}$ satisfies the equation $(A+\delta A) x_{1}=b$. (1) shows the important role of the condition number of $A$ for the discussion of the numerical stability, i.e., the importance of round-off errors, of methods for solving systems of linear equations.

Unfortunately the things are a little bit more complicated. Indeed suppose we solve the system by Gauss elimination without pivoting in binary floating-point arithmetic; it is easy to check that multiplications of the rows and of the columns by powers of 2 will not affect the relative precision of each component of the solution; however by this procedure, for a given norm, the condition number of the matrix can be made as large as one wishes. For this reason Bauer [2] has suggested that the real measure of the numerical stability of a system be defined by the optimal condition number:

$$
C_{\mathrm{op}}(A)=\inf _{D_{1}, D_{2} \in D} C\left(D_{1} A D_{2}\right)
$$

where $\mathfrak{D}$ is the set of regular diagonal matrices of order $N . C_{2}, C_{\text {op } 2}, C_{\infty}$ and $C_{0 p \infty}$ will denote the co ndition number and the optimal condition number for the euclidean and uniform nor ms. We recall that for symmetric matrices $C_{2}(A) \leqq C_{\infty}(A)$.

When the matrix $A$ is equilibrated, i.e. when the norms of the rows and columns
are of the some order, $C(A)$ and $C_{\mathrm{op}}(A)$ are not too different; several theorems make this statement precise (see [3]); we stall use the following one [4]: if $A$ is positive definite and possesses Young's property $A$ (in particuliar tridiagonal matrices have this property) and if all diagonal elements are equal, then $C_{2}(A)=C_{\text {op } 2}(A)$.

Let $L$ be an elliptic partial differential equation of order $2 m$ defined on a domain $G \subset R^{p}$. Let $L_{h}$ be the matrix obtained discretizing $L$ by finite differences on a regular net with step $h$ : suppose that stability and consistency are satisfied; they imply respectively the relations: $\left\|L_{h}^{-1}\right\|=O(1),\left\|L_{h}\right\|=O\left(h^{-2 m}\right)$ as $h \rightarrow 0$ and consequently $C\left(L_{h}\right)=O\left(h^{-2 m}\right)$. This result is independant of the dimension $p$ of $\boldsymbol{G}$; since the order $N$ of the matrix $L_{h}$ is proportionned to $h^{-p}$, it follows that $C\left(L_{h}\right)=O\left(N^{2 m / p}\right)$. For more precise statements about two-dimensional second order elliptic partial differential equations, see for example [5].

The main purpose of this talk is to discuss the numerical stability of matrices arising from the discretization of elliptic differential operators by the Ritz method. Let $G \subset R^{p}$ be a bounded domain, $V$ a closed subspace of real Hilbertian Sobolev space $H^{m}(G), \Lambda$ a bilinear form on $V \times V$ of the form $\Lambda(u, v)=$ $=\int_{G|\alpha!,,|\beta| \leqq m} a_{\alpha \beta}(x) D^{\alpha} u(x) D^{\beta} v(x) \mathrm{d} x$; one supposes $\Lambda(u, v)=\Lambda(v, u)$ and $\Lambda(u, v)>0$ for $u \neq 0$. Let $f_{1}, f_{2}, \ldots, f_{N}$ be independant elements spanning the subspace $U \subset V$. Let $H$ be the positive definite matrix of order $N$ with elements $\Lambda\left(f_{i}, f_{j}\right) ; H$ is called the stiffness matrix. We are interested in the condition number of $H$.

The following will show the importance of the degenerate case $m=0$. More precisely, besides $H$, we introduce the positive definite matrix $F$ of order $N$ with elements $\int_{G} f_{i}(x) f_{j}(x) \mathrm{d} x ; F$ is the matrix of the normal equations relative to the problem of least square approximation in the subspace $U ; F$ is called the mass matrix.

Consider first a classical example. $G=[0,1], f_{i}(x)=x^{i-1}, i=1,2, \ldots, N ; F$ is then the Hilbert matrix with $F_{i j}=1 /(i+j-1)$; the Hilbert matrix is well-known for its very large condition number, (see [1]); simple computations give the very optimistic lower bound: $C_{\infty}(F)>2^{2 N-3}$; in fact for $N=10, C_{2}(F)=1.610^{13}$. Equilibration cannot improve much the situation. On the other hand, simple examples for the bilinear form $\Lambda$ show that one cannot expect a better behaviour for the stiffness matrix. Because of the numerical instability it generates, this set of trial functions is not convenient; besides this, it presents notorious disadvantages: generally this a full matrix; general boundary conditions are difficult to satisfy. The remedy to these difficulties can be found in the method of finite elements.

We say that the set of functions $f_{1}, f_{2}, \ldots, f_{N}$ spanning $U \subset V$ is of finite element type if the following situation is present. Let $C_{i}$ be the support of $f_{i}$. One supposes the $C_{i}$ are small and one supposes the existence of sets $e_{1}, e_{2}, \ldots, e_{E}$ called elements with the following properties:

1) $G=\bigcup_{k=1}^{E} e_{k}$; measure $\left(e_{k}\right)>0$; measure $\left(e_{i} \cap e_{j}\right)=0 i \neq j$;
2) any $C_{i}$ is the union of a small number of elements; any element is covered by at least one $C_{i}$ :

We consider two simple examples for the bilinear form $\Lambda(u, v)=\int_{0}^{1} u^{\prime}(x) v^{\prime}(x) \mathrm{d} x$. First let $V=\left\{u \in H^{1}[0,1] ; u(0)=u(1)=0\right\}$; one divides $[0,1]$ in $N+1$ elements $e_{k}=\left[x_{k-1}, x_{k}\right] ; f_{i}, i=1,2, \ldots, N$ is the hat function of figure 1 . Second let $V=$ $=\left\{u \in H^{1}[0,1] ; u(0)=0\right\}$; one divides $[0,1]$ in $N$ elements $e_{k}=\left[x_{k-1}, x_{k}\right]$; for $i=1,2, \ldots, N-1 f_{i}$ is the hat function of figure $1 ; f_{N}$ is given by figure 2 . More general examples can be found in a very rich literature (see for example [6], [7]).


Figure 1


Figure 2

For the element $e_{k}$ let $I_{k}$ be the set of indices $j$ for which $C_{j} \supset e_{k}$; for $x \in R^{N}$ let $x^{k}$ be the subvector of $x$ corresponding to the set of indices $I_{k}$. One can write

$$
\begin{align*}
& x^{t} F x=\int_{G}\left(\sum_{i=1}^{N} x_{i} f_{i}\right)^{2}=\sum_{k=1}^{E} \sum_{i, j \in I_{k}}\left(\int_{e_{k}} f_{i} f_{j}\right) x_{i} x_{j}= \\
&=\sum_{k=1}^{E} x^{k t} F_{k} x^{k} ;  \tag{2}\\
& x^{t} H x=\Lambda\left(\sum_{i=1}^{N} x_{i} f_{i}, \sum_{j=1}^{N} x_{j} f_{j}\right)=\sum_{k=1}^{E} \sum_{i, j \in I_{k}}\left(\int_{e_{k}|\alpha|,|\beta| \leqq m} a_{\alpha \beta} D^{\alpha} f_{i} D^{\beta} f_{j}\right) x_{i} x_{j}= \\
&=\sum_{k=1}^{E} x^{k t} H_{k} x^{k} ; \tag{3}
\end{align*}
$$

$F_{k}$ and $H_{k}$ are symmetric matrices both of order equal to the number of elements of $I_{k}$; they are clearly defined by (2) and (3); they are called the mass and stiffness matrices of the element $e_{k}$. We suppose $F_{k}$ to be positive definite and $H_{k}$ to be semi positive definite. Let $\alpha_{k}$ and $w_{k}$ be the smallest and the largest eigenvalues of $F_{k}$ and let $\vartheta_{k}$ be the largest eigenvalue of $H_{k}$. Finally let $\alpha=\min _{k=1, \ldots, N} \alpha_{k}, w=\max _{k=1, \ldots, N} w_{k}$, $\vartheta=\max _{k=1, \ldots, N} \vartheta^{k}, \mu=w / \alpha$.

For the first example considered above $F_{k}$ and $H_{k}$ are given by (4) and (5) for $k=2,3, \ldots, N-1$ and by (6) for $k=1$ and $k=N$ (matrices of order 1 ); for the
second example (4) and (5) are valid for $k=2,3, \ldots, N$ and (6) is valid for $k=1$; in the following relations, we set $h_{k}=x_{k}-x_{k-1}$ :

$$
\begin{gather*}
F_{k}=\frac{h_{k}}{6}\left(\begin{array}{ll}
2 & 1 \\
1 & 2
\end{array}\right), \quad \alpha_{k}=\frac{h_{k}}{6}, \quad w_{k}=\frac{h_{k}}{2} ;  \tag{4}\\
H_{k}=\frac{1}{h_{k}}\left(\begin{array}{rr}
1 & -1 \\
-1 & 1
\end{array}\right), \quad \vartheta_{k}=\frac{2}{h_{k}} ;  \tag{5}\\
F_{k}=h_{k} / 3, \quad \alpha_{k}=w_{k}=h_{k} / 3 ; \quad H_{k}=1 / h_{k}, \quad \vartheta_{k}=1 / h_{k} . \tag{6}
\end{gather*}
$$

## 2. CONDITION NUMBER FOR THE EUCLIDEAN NORM

The results of this section are due to Isaac Fried [8], [9]. Let $P$ be the maximum number of elements contained in any of the supports $C_{i}$ of $f_{i}$ and let

$$
\lambda=\inf _{u \in V} \frac{(u, u)}{\int_{D} u^{2}}>0 .
$$

Theorem 1: $\|F\|_{2} \leqq P w,\left\|F^{-1}\right\|_{2} \leqq 1 / \alpha, \quad C_{2}(F) \leqq P w / \alpha$;

$$
\begin{equation*}
\|H\|_{2} \leqq P \vartheta, \quad\left\|H^{-1}\right\|_{2} \leqq 1 / \lambda \alpha, \quad C_{2}(F) \leqq P \vartheta / \lambda \alpha . \tag{7}
\end{equation*}
$$

Proof: From (2) one has for any $x \in R^{N}$ :

$$
\alpha x^{t} x \leqq \sum_{k=1}^{E} \alpha_{k} x^{k t} x^{k} \leqq \sum_{k=1}^{E} x^{k t} F_{k} x^{k}=x^{t} F_{x} \leqq \sum_{k=1}^{E} w_{k} x^{k t} x^{k} \leqq w P x^{t} x,
$$

which proves (7); the first inequality of (8) is obtained in the same way from (3); the second one is a consequence of the definition of $\lambda$; indeed for $u=\sum_{i=1}^{N} x_{i} f_{i}$ one has

$$
\lambda \alpha x^{t} x \leqq \lambda x^{t} F x=\lambda \int_{G} u^{2} \leqq \Lambda(u, u)=x^{t} H x .
$$

Example: One considers the first example described in section 1 on a regular net, i.e, $h_{k}=h=1 /(N+1) . P=2, \lambda=\pi^{2}, \alpha=h / 6, w=h / 2, \vartheta=2 / h$; theorem 1 gives the bounds:

$$
\begin{gathered}
\|F\|_{2} \leqq h, \quad\left\|F^{-1}\right\|_{2} \leqq 6 / h, \quad C_{2}(F) \leqq 6 ; \\
\|H\|_{2} \leqq 4 / h, \quad\left\|H^{-1}\right\|_{2} \leqq 6 /\left(\pi^{2} h\right), \quad C_{2}(H) \leqq 24 /\left(\pi^{2} h^{2}\right) ;
\end{gathered}
$$

direct computations show that $C_{2}(F) \sim 3, C_{2}(H)=4 /\left(\pi^{2} h^{2}\right)$ as $h \rightarrow 0$. The same asymptotic behaviour is also valid for almost uniform meshes; more specifically one considers a set of decompositions of [0, 1] in elements; for each decomposition let $h$ be the length of the largest element; one supposes the existence of a constant $\gamma$ independant of the decompositions such that for each decomposition the ratio
$h_{i} / h_{j}$ of the length of two elements is $\leqq \gamma$; then by theorem 1 one gets easily the results:

$$
C_{2}(F)=O(1), \quad C_{2}(H)=O\left(h^{-2}\right) \text { as } h \rightarrow 0 .
$$

Because the notion of finite element is not well defined, it is difficult to formulate a general theorem; however by using on the usual finite element functions the same technique as in the preceeding example, one gets for uniform and almost uniform meshes the asymptotical results:

$$
\begin{equation*}
C_{2}(F)=O(1), \quad C_{2}(H)=O\left(h^{-2 m}\right) \quad \text { as } \quad h \rightarrow 0 ; \tag{9}
\end{equation*}
$$

here $h$ denotes, for a particular decomposition of $D$, the maximum of the diameters of the elements; we recall that $m$ is the order of the bilinear form $\Lambda$; one must remark that the notion of "almost uniform mesh" is more complicated when the dimension $p$ of $G$ is $>1$ than for the one-dimensional case; for example, for decompositions in triangles, all the angles have to remain bounded above a fixed positive constant. It is interesting to note that (9) means that the asymptotic behaviour of the condition numbers of the discretizations matrices are the same for the finite element method and for the finite differences method.

## 3. CONDITION NUMBER FOR THE UNIFORM NORM

The asymptotic results are essentially the same as for the euclidean norm, but less general and more complicated to obtain. The following theorems are proved in [11]; other results are contained in [10].

Besides the notations of section 1, we introduce the following ones. For a subset $Z \subset G, m(Z)$ is its measure, $d(Z)$ its diameter; $c_{p}$ is the measure of the unit sphere in $\boldsymbol{R}^{p}$; let $M=$ maximum number of supports $C_{i}$ covering a same element;

$$
\begin{gathered}
\frac{d^{p}\left(C_{j}\right)}{m\left(e_{k}\right)} \leqq \gamma, \quad i=1, \ldots, N, k=1, \ldots, E ; \\
\frac{d^{p}\left(e_{i}\right)}{d^{p}\left(e_{i}\right)} \leqq \delta, \quad i, j=1,2, \ldots, E .
\end{gathered}
$$

Theorem 2.

$$
\left\|F^{-1}\right\|_{\infty} \leqq s^{-1} \alpha^{-1}\left(M c_{p} n^{p} \gamma\right)^{\frac{1}{2}},
$$

where $s$ is any number between 0 and 1 and $n$ is the smallest integer for which

$$
\mu^{-2} M c_{p} \gamma p(1-\mu)^{n-1} n^{p-1} \leqq(1-s)^{2} .
$$

Theorem 3. One supposes that $\Lambda$ satisfies the following coerciveness relation

$$
\Lambda(u, u) \geqq x\left(\int_{G} u^{2}\right)^{\frac{1}{2}} \max _{x \in G}|u(x)|, \quad u \in V, \quad x>0 ;
$$

then $\left\|H^{-1}\right\|_{\infty} \leqq \mathcal{X}^{-1}\left(c_{p} 2^{-p} \mu^{-2} \delta M m(D)\right)^{\frac{1}{2}}\left\|F^{-1}\right\|$.

The simplest way to evaluate $\|F\|_{\infty}$ and $\|H\|_{\infty}$ is by direct inspection of the matrices; however one can also use the bounds of theorem 1:

$$
\begin{equation*}
\|F\|_{\infty} \leqq \sqrt{Q}\|F\|_{2}, \quad\|H\|_{\infty} \leqq \sqrt{Q}\|H\|_{2}, \tag{10}
\end{equation*}
$$

where $Q$ is the maximum number of supports $C_{i}$ having a intersection of positive measure.

We give a brief proof of (10). Let $A$ be $F$ or $H$; each row of $A$ has at most $Q$ elements different from zero; for the row $i$ let $I$ be the set of indices $j$ with $a_{i j} \neq 0$; let $A^{*}$ be the square submatrix of order $\leqq Q$ corresponding to $I$; for $x \in \boldsymbol{R}^{N}$ let $x^{*}$ be the subvector corresponding to $I$; one has

$$
\begin{gathered}
\left|(A x)_{i}\right| \leqq\left\|A^{*} x^{*}\right\|_{\infty} \leqq\left\|A^{*} x^{*}\right\|_{2} \leqq\left\|A^{*}\right\|_{2}\left\|x^{*}\right\|_{2} \leqq\left\|A^{*}\right\|_{2} \sqrt{Q}\left\|x^{*}\right\|_{\infty} \leqq \\
\leqq\|A\|_{2} \sqrt{Q}\|x\|_{\infty} .
\end{gathered}
$$

Example: Again we consider the first example described in section 1 with a uniform mesh, i.e., $h_{k}=h=1 /(N+1), k=1,2, \ldots, N+1$. We have $p=1, c_{p}=2, M=$ $=2, \gamma=2, \delta=1, \alpha=h / 6, \mu=1 / 3, \chi=2 \pi, Q=3$; theorem 2 with $s=0.75$, theorem 3 and relation (10) give

$$
\begin{gathered}
\|F\|_{\infty} \leqq 1.73 / h, \quad\left\|F^{-1}\right\|_{\infty} \leqq 98.5 / h, \quad C_{\infty}(F) \leqq 170 ; \\
\|H\|_{\infty} \leqq 5.92 / h, \quad\left\|H^{-1}\right\|_{\infty} \leqq 66.5 / h, \quad C_{\infty}(H) \leqq 394 / h^{2} ;
\end{gathered}
$$

direct computations show that $2.8 \leqq C_{\infty}(F) \leqq 3$ and, as $h \rightarrow 0, C_{\infty}(H) \sim 1 /\left(2 h^{2}\right)$.
As for the euclidean norm, the method of finite elements leads for uniform and almost uniform meshes to the results

$$
C_{\infty}(F)=O(1), \quad C_{\infty}(H)=O\left(h^{-2 m}\right) \quad \text { as } \quad h \rightarrow 0 ;
$$

we have to emphasize the fact that this last result relative to $H$ supposes that the coerciveness condition of theorem 3 is satisfied; it is a conjecture that it should be possible to relax considerably this restriction.

## 4. FINITE ELEMENTS ON NON UNIFORM MESHES

In [9] Fried considers decompositions of $G$ with the presence of two adjacent elements having very different sizes. Since the mass and stiffness matrices are not equilibrated in this case, the discussion of their numerical stability supposes a proper scaling. With the help of various examples, Fried shows that in some cases the numerical stability of the stiffness matrix is as good as in the case of a uniform mesh but in other cases it can be much worse.

Here we adopt a different point of view which will lead to similar results for the stiffness matrix. We consider a set of decompositions of the domain $G$ in elements; for the sake of simplicity we suppose that all the elements contained in any support
$C_{i}$ have a point in common; we also suppose the existence of the numbers $\alpha^{*}, \beta^{*}, \gamma^{*}$ independant of the decompositions such that for each decomposition one has

$$
\begin{gather*}
\alpha^{*} m\left(e_{k}\right) x^{k t} x^{k} \leqq x^{k t} F_{k} x^{k} \leqq \omega^{*} m\left(e_{k}\right) x^{k t} x^{k},  \tag{11}\\
m\left(e_{i}\right) \leqq \gamma^{*} m\left(e_{j}\right) \quad \text { if } \quad e_{i} \cap e_{j} \neq 0 \tag{12}
\end{gather*}
$$

$m\left(e_{i}\right)$ denotes the measure of $e_{i} ; x^{*}$ and $F_{k}$ have been defined in section 1. (12) means that, in a decomposition, two elements having a point in common cannot be too different in size. (11) is satisfied by the usual finite elements. Consider a particular decomposition and a support $C_{i}$; let $\mathscr{E}$ be the set of the elements contained in $C_{i}$ and $n$ be the number of elements of $\mathscr{E} ; q_{i}=\left(\sum_{e \in \mathscr{E}} m(e)\right) / n$ is the average measure of the elements in $\mathscr{E}$; finally let $D$ be the diagonal matrix of order $N$ with diagonal elements $q_{i}^{-\frac{1}{2}}$ and $D_{k}$ be the diagonal submatrix of $D$ relative to the indices of $I_{k}$ (see definition in section 1); we introduce the vector $y$ of order $N$ with components $y_{i}$ and the subvector $y^{k}$ defined by the relations

$$
x=D y, \quad x_{i}=y_{i} / \sqrt{q_{i}}, \quad x^{k}=D_{k} y^{k} ;
$$

if $i \in I_{k}$; then $\gamma^{*-1} \leqq m\left(e_{k}\right) / q_{i} \leqq \gamma^{*}$; replacing in (11) we get

$$
\alpha^{*} \gamma^{*-1} y^{k t} y^{k} \leqq \alpha^{*} m\left(e_{k}\right) y^{k t} D_{k}^{2} y^{k} \leqq y^{k t} D_{k} F_{k} D_{k} y^{k} \leqq \omega^{*} m\left(e_{k}\right) y^{k t} D_{k}^{2} y^{k} \leqq \omega^{*} \gamma^{*} y^{k t} y^{k} ;
$$

the arguments used in theorem 2 and the relation

$$
\sum_{k=1}^{E} y^{k t} D_{k} F_{k} D_{k} y^{k}=y^{t} D F D y
$$

prove the following result:

$$
\begin{equation*}
\|D F D\|_{2} \leqq P \omega^{*} \gamma^{*} ; \quad\left\|(D F D)^{-1}\right\|_{2} \leqq \gamma^{*} / \alpha^{*} ; \quad C_{2}(D F D) \leqq P \omega^{*} \gamma^{* 2} / \alpha^{*} \tag{13}
\end{equation*}
$$

so we have proved that $C_{\text {op } 2}(F)$ is bounded by a constant independant of the decompositions.

As an illustration we take the second example of section 1 (boundary condition $u(0)=0)$ with $h_{k}=a^{k-1}(1-a) /\left(1-a^{N}\right), a<0$; we have $\gamma^{*}=1 / a, m\left(e_{k}\right)=h_{k}$, $\alpha^{*}=1 / 6, \omega^{*}=1 / 2, P=2$; from (13) we get $C_{\mathrm{op} 2}(F) \leqq 6 / a^{2}$; in fact direct computations show that $\lim _{a \rightarrow 0} C_{\text {ov2 }}(F)=1$ uniformely in $N$.
(13) is a very satisfactory result for the mass matrix. The following three examples show that it is not possible to get simple results for the stiffness matrix.
a) We consider example 1 of section $\mathbf{1}$ (boundary condition $u(0)=u(1)=0$ ) with $h_{k}=a^{k-1}(1-a) /\left(1-a^{N+1}\right), k=1,2, \ldots, N+1, a<1$. Direct computations and the property stated in section 1 on optimal conditionning give the following result

$$
C_{\mathrm{op} 2}(H) \leqq C_{\mathrm{op} \infty}(H) \leqq\left(\frac{1+\sqrt{a}}{1-\sqrt{a}}\right)^{2} \quad \text { (independantly of } N \text { ); }
$$

we recall that for $a=1$ we got in section 2: $C_{\text {op } 2}(H) \sim 4 N^{2} / \pi^{2}$ as $N \rightarrow \infty$; in particular we have the surprising result: $\lim _{n \rightarrow 0} C_{\text {op } \delta}(H)=1$ uniformely in $N$.
b) We consider example 2 of section 1 (boundary condition $u(0)=0$ ) with $h_{k}=$ $=a^{k-1}(1-a) /\left(1-a^{N}\right), k=1,2, \ldots, N, a<1$. By direct computation we get the following result

$$
a^{-N}\left\{\frac{2 a(1+\sqrt{a})^{2}}{(1+a)(1-a)^{2}}-\varepsilon_{N}\right\} \leqq C_{\mathrm{op} 2}(H) \leqq a^{-N}\left\{\frac{2 a\left\{(1+\sqrt{a})^{2}+1\right\}}{(1+a)(1-a)^{2}}+\varepsilon_{N}\right\}
$$

with $\lim _{N \rightarrow \infty} \varepsilon_{N}=0$; we have therefore $C_{\text {op } 2}(H)=O\left(a^{-N}\right)$ whereas for $a=1$ theorem 1 gives $C_{2}(H)=O\left(N^{2}\right)$.
c)*) We consider example 1 of section 1 (boundary condition $u(0)=u(1)=0$ ) for $N$ odd, $\quad N+1=2 q, \quad h_{k}=h_{2 q+1-k}=a^{k-1}(1-a) /\left(1-a^{q}\right), \quad k=1,2, \ldots, q$ (figure 3); the elements are concentrated around $x=0.5$. Denoting by $C_{b}(N)$ the optimal condition number obtained for $H$ in the preceeding example $b$, one gets easily the following relations

$$
0.5 C_{b}(q) \leqq C_{\text {op } 2}(H) \leqq 2 C_{b}(q) ;
$$

therefore $C_{\text {op } 2}(H)=O\left((\sqrt{a})^{-N}\right)$ whereas for $a=1$ we have $C_{2}(H) \sim 4 N^{2} / \pi^{2}$ as $N \rightarrow \infty$.

Remark: Instead of computing the asymptotic growth of $C_{o p}(H)$ with respect to $N$, we can consider it with respect to the length $h_{\text {min }}$ of the smallest element; for examples b) and c) we then have the comforting results

$$
C_{\mathrm{op} 2}(H)=O\left(h_{\min }^{-1}\right) \quad \text { and } \quad C_{\mathrm{op} 2}(H)=O\left(h_{\min }^{-1}\right) .
$$



Figure 3

## 5. CONCLUDING REMARKS

1. In [13] Fix and Strang have obtained the results of sections $\mathbf{2}$ and $\mathbf{3}$ for uniform meshes by using Fourier transforms.
2. Since results on the condition numbers of stiffness matrices are essentially equivalent to the usual stability properties for the finite differences method, it is possible to deduce from them results on convergence. However for the finite element method properties of consistency are not easy to establish.

[^0]3. One can use the results on the condition number of stiffness matrices for studying perturbation problems, for example the effect of numerical integration in the computation of the elements of the stiffness matrix; however one does not get optimal results in this way (see [14]).

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